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A-61004

 THE PORT AUTHORITY OF NEW YORK AND NEW JERSEY
 ENGINEERING COMPUTATIONS
NAME OF STRUCTURE DEY STREET UNDERPASSSHEET NO. 1 OF 3SUBJECT MATTER SEWER DESIGNMADE BY J. SPENCER DATE 11/30/99

CHECKED BY _____ DATE _____

I DETERMINE STORM WATER FLOW:

$$Q \text{ (cfs)} = C \times R \text{ (in/hr)} \times A \text{ (AREA)}$$

A. COMPUTE THE CONTRIBUTARY AREA - "A" - SEE ATTACHMENT 1.

NO RUNOFF AT STREET LEVEL ENTERS THE DRY STREET SEWER FROM BROADWAY TO CHURCH STREET

AS SHOWN ON ATTACHMENT 1, ONLY ROOF RUNOFF IS ASSUMED TO ENTER THE DRY STREET SEWER

$$\begin{aligned} \therefore \text{AREA} &= 70' \times 350' && \text{ROOF AREA - DRY TO FULTON ST} \\ &= 24,500 \text{ ft}^2 \\ &100' \times 330' && \text{ROOF AREA - DRY TO MARIEN LANE} \\ &= 33,000 \text{ ft}^2 \end{aligned}$$

$$\begin{aligned} \text{TOTAL AREA} &= 57,500 \text{ ft}^2 / 43,560 \text{ ft}^2/\text{ACRE} \\ &= 1.32 \text{ ACRES} \end{aligned}$$

B. COMPUTE THE RAINFALL INTENSITY "R" & RUNOFF COEFFICIENT "C"

$$R = \frac{125}{T+15}$$

T = TOTAL TIME OF CONCENTRATION IN MINUTES TO THE UPPER END OF THE SEWER

ENTRANCE TIME = 6 MINUTES
LENGTH OF SEWER = 340' IN DEY STREET
VELOCITY OF FLOW - ASSUME 3'/SEC. AS A MINIMUM

$$\therefore \text{RUNNING TIME IN SEWER} = 340' / 3 = 113 \text{ SEC OR } 1.89 \text{ MIN}$$

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SUBJECT MATTER SEWER DESIGN MADE BY J. SPENCER DATE 11/30/79
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$$\therefore R = \frac{125}{6 + 1.89 + 15} = 5.46 \text{ IN/HR}$$

$$\text{USE } C = .90$$

$$\therefore Q = CRA$$

$$(.9)(5.46)(1.32) = 6.49 \text{ CFS}$$

II DETERMINE SANITARY FLOW

ASSUME DISCHARGE INTO DEY STREET FROM BUILDINGS ON BOTH SIDES OF THE STREET

A.T. & T. IS THE MAJOR TENANT ON THE NORTH SIDE OF DEY STREET

POPULATION OF A.T. & T. BUILDING IS 2600 PEOPLE

THE A.T. & T. DISCHARGE IS INTO BOTH DEY & FULTON STREET.

FOR DOMESTIC FLOW, USE 150 GALLONS PER CAPITA PER DAY

$\therefore$ AVERAGE FLOW FROM A T & T BUILDING IS

$$1300 \text{ PEOPLE} \times 150 \text{ GAL/PERSON/DAY}$$

$$= 195,000 \text{ GPD (AVERAGE)}$$

ASSUME SAME DISCHARGE INTO DEY STREET SEWER FROM THE BUILDINGS ON THE SOUTH SIDE $\therefore$ AVERAGE FLOW = 390,000 GPD

PEAK FLOW = AVERAGE FLOW $\times$ PEAK FACTOR

USING A PEAK FACTOR OF 2.0

$$\text{PEAK FLOW} = 390,000 \text{ GPD} \times 2.0 = 780,000 \text{ GPD} = .78 \text{ MGD}$$

$$.78 \text{ MGD} \times .646317 = 1.21 \text{ CFS}$$

THE PORT AUTHORITY OF NEW YORK AND NEW JERSEY

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NAME OF STRUCTURE DEY STREET UNDERPASSSHEET NO. 3 OF 3SUBJECT MATTER SEWER DESIGNMADE BY J. SPENCER DATE 11/20/79

CHECKED BY _____ DATE _____

III CHECK TO SEE IF 15" ϕ PIPE IS CAPABLE OF HANDLING TOTAL COMBINED FLOW

$$\text{COMBINED FLOW} = 6.49 + 1.21 = 7.70 \text{ CFS}$$

15" ϕ R.C.P. MANNING COEFFICIENT $n = .013$

FROM PROFILE OF NEW SEWER

$$S = \frac{308.63 - 307.71}{27'} = .0341$$

USE MANNING FORMULA TO COMPUTE THE CAPACITY OF THE SEWER

$$Q = \frac{1.486}{n} A R^{2/3} S^{1/2}$$

Q = CAPACITY IN CFS

n = ROUGHNESS COEFFICIENT

A = CROSS SECTIONAL AREA OF WATER IN PIPE IN S.F.

R = HYDRAULIC RADIUS IN FEET = $d/4$

S = SLOPE OF SEWER

FOR 15" ϕ

$$A = \pi r^2$$

$$= \pi (625')^2 = 1227 \text{ FT}^2$$

$$R = d/4 = .3125 \text{ FT}$$

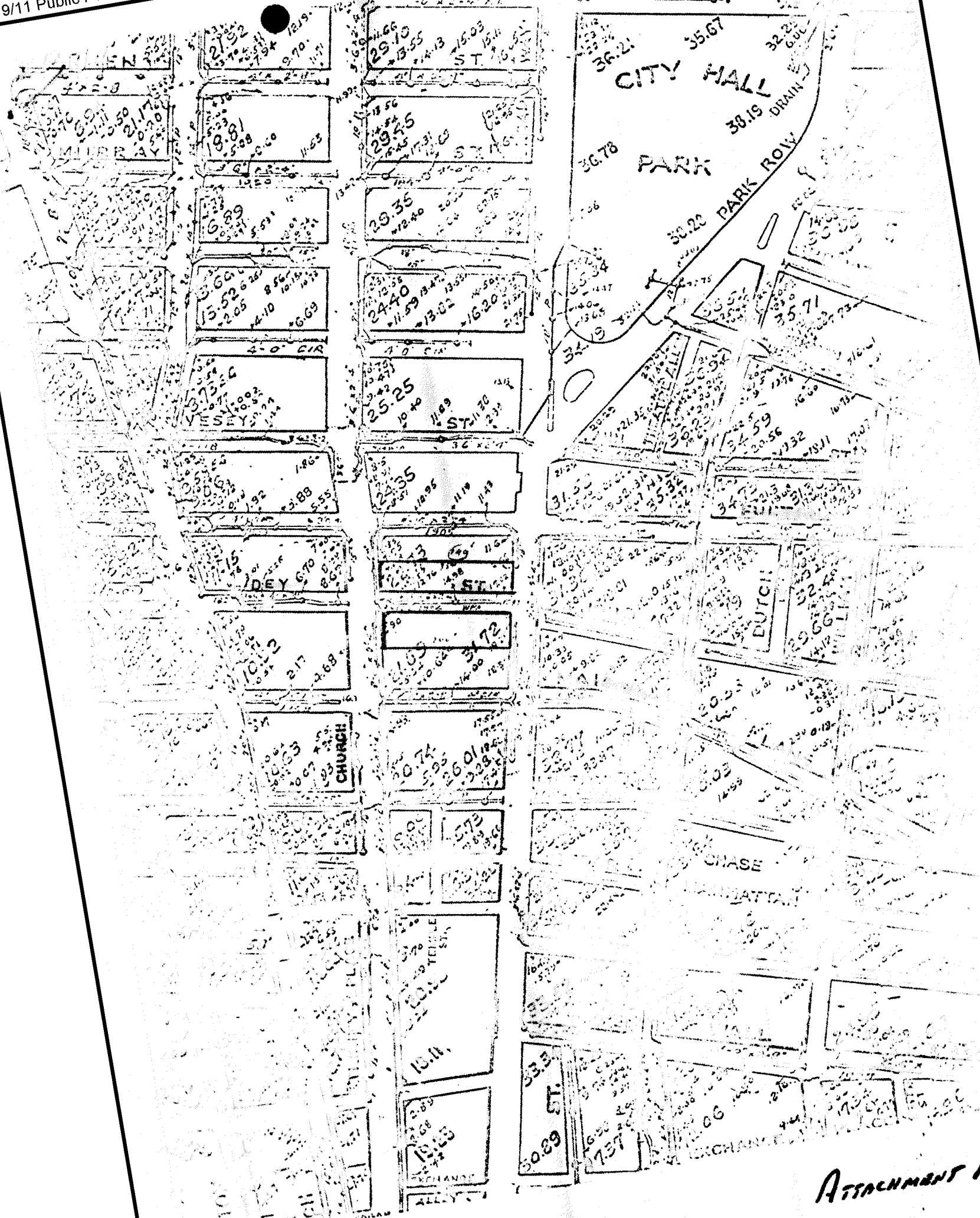
$$R^{2/3} = .4605 \text{ FT}$$

$$S = .0341$$

$$S^{1/2} = .1847$$

$$\therefore Q = \frac{1.486}{.013} \times 1227 \text{ FT}^2 \times .4605 \times .1847 = 11.92 \text{ CFS}$$

11.92 CFS > 7.70 CFS $\therefore$ 15" ϕ PIPE IS O.K.



MANHATTAN
SEWER INDEX
SHEET 1 of 8

Attachment 1.
SCALE: 1"=300'